CHAPTER 1: Analysis of Beams

### 1.3 Single Span Beams Shear and Moment

## Description

This application computes the reactions and the maximum bending moment, and plots the shear and bending moment for a single span beam, with or without end moments, loaded with any practical number of uniformly distributed and concentrated loads. The user must divide the beam into segments with each segment supporting a single uniformly distributed load over its length and/or a concentrated load at the right end of each segment.

The user must enter the length of each segment, the uniformly distributed loads on each segment, the concentrated loads at the right end of each segment, and the left and right end moments.

A summary of input and calculated values is shown on pages 8 .

The user should be familiar with subscript notation, entering numbers as vectors, and using the transpose and vectorize operators on the palette.

Notation
$n$ is the last
segment


## Input Variables

a segment lengths
w uniformly distributed loads on segments
P concentrated loads at right end of segments
ML left end moment
Mr right end moment

## Computed Variables

The following variables are calculated in this document:

L span length
$R_{L}$ beam reaction at left end
$\mathrm{R}_{\mathrm{R}}$ beam reaction at right end
XL distance from the left reaction to the point of maximum moment
$M_{\text {max }}$ maximum bending moment
$\mathrm{V}(\mathrm{x})$ shear at distance x from left reaction
$\mathrm{M}(\mathrm{x})$ bending moment at distance x from right reaction

## Defined Units

$$
p l f:=\frac{l b f}{f t}
$$

ORIGIN set equal to 1 to agree with customary usage. ORIGIN is a PTC Mathcad variable, the index number of the first element of a vector or matrix.

ORIGIN : = 1

## Input

Enter segment lengths, uniformly distributed loads, and concentrated loads starting from the left reaction, and the left and right end moments:

| Segment lengths: | $a:=\left[\begin{array}{lll}3.5 & 10 & 13.5\end{array}\right]^{\mathrm{T}} \cdot f t$ |
| :---: | :---: |
| Span length: | $L:=\sum a \quad L=27 \mathrm{ft}$ |
| Uniformly distributed loads: | $w:=\left[\begin{array}{lll}2.6 & 1.8 & 0.5\end{array}\right]^{\mathrm{T}} \cdot \frac{k i p}{f t}$ |
| Concentrated loads at the right end of segments: | $p:=\left[\begin{array}{ll}7.8 & 10.3\end{array}\right]^{\mathrm{T}} \cdot \mathrm{kip}$ |
| Left end moment: | $M_{L}:=97.5 \cdot k i p \cdot f t$ |
| Right end moment: | $M_{R}:=121 \cdot k i p \cdot f t$ |

## Calculations

Beam reactions, location of the point of zero shear, and maximum bending moment are computed within this section.

Maximum number of segments entered: $n:=\operatorname{rows}(a) \quad n=3$

The following expressions adjusts the sizes of vectors P and w to the same size as vector a :

$$
\begin{array}{ll}
P:=p & P_{n}:=\operatorname{if}\left(\operatorname{rows}(p)<\operatorname{rows}(a), 0 \cdot k i p, p_{n}\right) \\
W:=w & W_{n}:=\operatorname{if}\left(\operatorname{rows}(w)<\operatorname{rows}(a), 0 \cdot \frac{l b f}{f t}, w_{n}\right)
\end{array}
$$

Range variable i from 1 to $n$; range variable i1 from 2 to $n$ :

$$
i:=1 \ldots n \quad i 1:=2 \ldots n
$$

Sum of the loads to the left side of each segment:

$$
\begin{aligned}
& V L_{1}:=0 \cdot k i p \quad V L_{i 1}:=V L_{i 1-1}+W_{i 1-1} \cdot a_{i 1-1}+P_{i 1-1} \\
& V L^{\mathrm{T}}=\left[\begin{array}{lll}
0 & 16.9 & 45.2
\end{array}\right] k i p
\end{aligned}
$$

Sum of the loads to the right side of each segment:

$$
\begin{aligned}
& V R_{1}:=W_{1} \cdot a_{1} \quad V R_{i 1}:=V R_{i 1-1}+P_{i 1-1}+W_{i 1} \cdot a_{i 1} \\
& V R^{\mathrm{T}}=\left[\begin{array}{ll}
9.1 & \ldots
\end{array}\right] \text { kip }
\end{aligned}
$$

Sum of the moments due to loads, at the right end of each segment:

$$
\begin{aligned}
& M R_{1}:=\frac{W_{1} \cdot\left(a_{1}\right)^{2}}{2} \quad M R_{i 1}:=M R_{i 1-1}+\frac{V L_{i 1}+V R_{i 1}}{2} \cdot a_{i 1} \\
& M R^{\mathrm{T}}=[15.9 \ldots] \mathrm{kip} \cdot \mathrm{ft}
\end{aligned}
$$

Left end reaction:

$$
R_{L}:=\frac{M R_{n}}{\sum a}+\left(\frac{M_{L}-M_{R}}{L}\right) \quad R_{L}=33.6 \mathrm{kip}
$$

Right end reaction:

$$
R_{R}:=\sum_{i}\left(W_{i} \cdot a_{i}+P_{i}\right)-R_{L} \quad R_{R}=18.35 \mathrm{kip}
$$

Shear at the left end of each segment:

$$
\begin{aligned}
& V_{L_{i}}:=R_{L}-V L_{i} \\
& V_{L}^{\mathrm{T}}=\left[\begin{array}{lll}
33.6 & 16.7 & -11.6
\end{array}\right] \mathrm{kip}
\end{aligned}
$$

Shear at the right end of each segment:

$$
\begin{aligned}
& V_{R_{i}}:=R_{L}-V R_{i} \\
& V_{R}^{\mathrm{T}}=\left[\begin{array}{ll}
24.5 & \ldots
\end{array}\right] \mathrm{kip}
\end{aligned}
$$

Moments at the left end of each segment:

$$
\begin{aligned}
& M_{s_{1}}:=-M_{L} \\
& M_{s_{i 1}}:=M_{s_{i 1-1}}+\frac{1}{2} \cdot\left(V_{L_{i 1-1}}+V_{R_{i 1-1}}\right) \cdot a_{i 1-1} \\
& M_{s}^{\mathrm{T}}=\left[\begin{array}{lll}
-97.5 & \ldots
\end{array}\right] \text { kip } \cdot f t
\end{aligned}
$$

Matrix U with elements equal to 1 if the corresponding element in $\mathrm{V}_{\mathrm{L}}$ is greater than or equal to 0 kip or with elements equal to 0 if the corresponding element is less than 0 kip:

$$
\begin{aligned}
& U_{i}:=\operatorname{if}\left(V_{L_{i}}>0 \cdot k i p, 1,0\right) \\
& U^{\mathrm{T}}=\left[\begin{array}{lll}
1 & 1 & 0
\end{array}\right]
\end{aligned}
$$

Index of the segment where shear passes through zero:

$$
u:=\left(\sum U\right) \quad u=2
$$

Distance from the left end of the segment where shear passes through 0 to the point where shear passes through 0 :

$$
a^{\prime}:=\operatorname{if}\left(W_{u}=0 \cdot \frac{k i p}{f t}, a_{u}, \text { if } \left.\left(\frac{V_{L_{u}}}{W_{u}}>a_{u}, a_{u}, \frac{V_{L_{u}}}{W_{u}}\right) \right\rvert\,\right) \quad a^{\prime}=9.278 \mathrm{ft}
$$

Distance from the left reaction to the left end of each segment:

$$
\begin{aligned}
& S_{L_{1}}:=0 \cdot f t \quad S_{L_{i 1}}:=S_{L_{i 1-1}}+a_{i 1-1} \\
& S_{L}{ }^{\mathrm{T}}=\left[\begin{array}{lll}
0 & 3.5 & 13.5
\end{array}\right] \mathrm{ft}
\end{aligned}
$$

Distance from the left end reaction to the point of zero shear and maximum moment:

$$
X_{L}:=S_{L_{u}}+a^{\prime} \quad X_{L}=12.778 \mathrm{ft}
$$

Maximum bending moment:

$$
\begin{aligned}
& M_{\text {max }}:=M_{s_{u}}+\left(V_{L_{u}} \cdot a^{\prime}-\frac{1}{2} \cdot W_{u} \cdot a^{\prime^{2}}\right) \\
& M_{\text {max }}=81.639 \mathrm{kip} \cdot \mathrm{ft}
\end{aligned}
$$

Vectors V and xv for plotting shear diagram:

$$
\begin{array}{ll}
V_{2 \cdot i-1}:=V_{L_{i}} & V_{2 \cdot i}:=V_{R_{i}} \\
x_{v_{2 \cdot i-1}}:=S_{L_{i}} & x_{v_{2 \cdot i}}:=S_{L_{i}}+a_{i}
\end{array}
$$

Range variable i2 for plotting shear diagram:

$$
i 2:=1 . .2 \cdot n
$$

Number of equally spaced points for plotting moment diagram:

$$
N:=100
$$

Note $\Rightarrow \mathrm{N}$ may be any reasonable number greater than or equal to 1.

Range variable i3 and equally spaced points xm for plotting moment diagram:

$$
i 3:=1 . . N \quad x_{m_{i 3}}:=i 3 \cdot \frac{L}{N}
$$

Distances to the left end of each segment added to vector Xm:

$$
x_{m}:=\left(\operatorname{augment}\left(x_{m}{ }^{\mathrm{T}}, S_{L}{ }^{\mathrm{T}}\right)^{\mathrm{T}}\right)
$$

Distance to the point of zero shear added as last element in vector xm:

$$
x_{m_{N+n+1}}:=X_{L}
$$

Range variable i4 for all points to be plotted:

$$
i 4:=1 . . N+n+1
$$

Sort the elements of vector xm in ascending order:

$$
x_{m}:=\operatorname{sort}\left(x_{m}\right)
$$

Moment at distance x from the left reaction, as a function of x :
$M(x):=\sum_{i}\left(\left(\left(x>S_{L_{i}}\right) \cdot\left(x \leq\left(S_{L_{i}}+a_{i}\right)\right)+(i=1) \cdot(x=0 \cdot f t)\right) \cdot\left(M_{s_{i}}+V_{L_{i}} \cdot\left(x-S_{L_{i}}\right)-\frac{1}{2} \cdot W_{i} \cdot\left(x-S_{L_{i}}\right)^{2}\right)\right)$

## Summary

## Input Variables

Segment lengths: $\quad a^{T}=\left[\begin{array}{lll}3.5 & 10 & 13.5\end{array}\right] f t$

Uniformly
distributed loads: $\quad W^{T}=\left[\begin{array}{lll}2.6 & 1.8 & 0.5\end{array}\right] \frac{\mathrm{kip}}{\mathrm{ft}}$

Concentrated loads at right end of $\quad P^{T}=\left[\begin{array}{lll}7.8 & 10.3 & 0\end{array}\right]$ kip segments:

Left end moment: $\quad M_{L}=97.5 \mathrm{kip} \cdot f t$

Right end moment: $\quad M_{R}=121 \mathrm{kip} \cdot \mathrm{ft}$

## Computed Variables

| Span <br> lengths: | Left <br> Reactions: | Right <br> Reactions: |
| :--- | :--- | :--- |
| $L=27 \mathrm{ft}$ | $R_{L}=33.6 \mathrm{kip}$ | $R_{R}=18.35 \mathrm{kip}$ |

Distances from the left reaction to the point of zero shear: $\quad X_{L}=3.895 \mathrm{~m}$
Maximum moment: $\quad M_{\max }=81.639 \mathrm{kip} \cdot \mathrm{ft}$


Note $\Rightarrow$ PTC Mathcad plots may be enlarged to show small scale detail.

